



East Devon District Council Level 2 Strategic Flood Risk Assessment Detailed Site Summary Tables





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**Site Code****Brc1_31****Address**

Land to the east of the M5, West Clyst

**Surface water
plus climate
change**

Available data and mapping: Environment Agency's Risk of Flooding from Surface Water dataset for the 3.33%, 1% and 0.1% AEP events with 65% Climate Change scenarios.

Brc1_31 - Surface Water 3.33% AEP plus 65% Climate Change - Depth
 Brc1_31 - Surface Water 3.33% AEP plus 65% Climate Change - Hazard
 Brc1_31 - Surface Water 3.33% AEP plus 65% Climate Change - Velocity
 Brc1_31 - Surface Water 1% AEP plus 65% Climate Change - Depth
 Brc1_31 - Surface Water 1% AEP plus 65% Climate Change - Hazard
 Brc1_31 - Surface Water 1% AEP plus 65% Climate Change - Velocity
 Brc1_31 - Surface Water 0.1% AEP plus 65% Climate Change - Depth
 Brc1_31 - Surface Water 0.1% AEP plus 65% Climate Change - Hazard
 Brc1_31 - Surface Water 0.1% AEP plus 65% Climate Change - Velocity

Management Catchment: Brc1_31 is located within the East Devon Management Catchment. The Environment Agency guidance recommends that the Upper End allowance is considered for both the 3.3% and 1% AEPs for the 2070's epoch, unless the allowance for the 2050's epoch is higher, in which case this should be used. This is appropriate for development with a lifetime beyond 2100. The recommended uplift on peak rainfall intensity for the 3.3% AEP is 40% and for the 1% AEP is 45%. As Risk of Flooding from Surface Water data with a 65% uplift was already available this has been used as the best available data for the 3.3%, 1% and 0.1% AEPs.

Data analysis:**3.3% AEP (1 in 30 year) plus 65% climate change event:**

Proportion - 8%	
Max Depth - 1.97m	Mean Depth - 0.31m
Max Velocity - 2.61m/s	Mean Velocity - 0.28m/s
Max Hazard - 3.24	Mean Hazard - 0.96

1% AEP (1 in 100 year) plus 65% climate change event:

Proportion - 11%	
Max Depth - 2.11m	Mean Depth - 0.37m
Max Velocity - 2.8m/s	Mean Velocity - 0.36m/s
Max Hazard - 4.42	Mean Hazard - 1.07



0.1% AEP (1 in 1000 year) plus 65% climate change event:


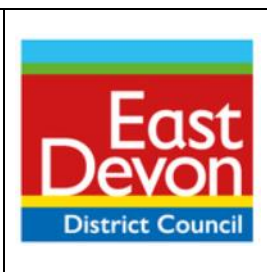
Proportion - 17%	
Max Depth - 2.62m	Mean Depth - 0.71m
Max Velocity - 5.13m/s	Mean Velocity - 0.52m/s
Max Hazard - 8.97	Mean Hazard - 1.41



Flood characteristics: The site is shown to be at risk of flooding in all three scenarios with flooding along the north, east and southern boundaries. The mean depth is shown to be 0.71m in the 0.1% AEP event plus 65% climate change. The average velocity on site is 0.52m/s, with a maximum of 5.13m/s. The average hazard rating during the 1% AEP plus 65% climate change event is 1.07 and is therefore stated to be a 'danger to some'.

Reservoir

The site is not located near to a Wet or Dry day reservoir flooding extent, according to the Environment Agency's reservoir flood mapping.

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Groundwater	<p>Available data and mapping: The JBA Groundwater Flood Data Map (GW5) is provided as a 5m resolution grid.</p> <p>Brcl_31 - Groundwater Emergence</p> <p>Flood characteristics: During a 1% AEP groundwater flood event, groundwater levels on site are mixed and comprise of all levels of groundwater from: at or very near (within 0.025m of) the ground surface, to levels at least 5m below the surface, or at low risk. The highest risk of groundwater flooding is predominantly located within the southwest and southeast of the site, with the lowest risk in the north.</p> <p>Flow paths would be expected to follow the topography of the site and be similar to surface water flow paths. The risk of groundwater flooding will require further consideration in a site-specific flood risk assessment.</p>	
Sewers	No evidence of sewer flooding has been identified at or near the development site.	
Flood history	<p>Available data and mapping: Environment Agency's Recorded Flood Outlines.</p> <p>Flood characteristics: A small portion along the north, east and southern boundary of the site are shown to be located within the Environment Agency's Recorded Flood Outlines extent. These events are all due to the River Clyst exceeding capacity in 1965, 1972, 1980 and 1992.</p> <p>There are no flooding incidents within Devon County Council's dataset recorded within 100m of the site.</p>	
Policy zones		
Critical drainage areas	<p>The site has not been identified to be located within a critical drainage area.</p> <p>Mapping: Brcl_31 - Critical Drainage Area</p>	
Coastal change management areas	The site has not been identified to be located within a coastal change management area.	
Flood risk management infrastructure		
Existing defences	The Environment Agency's AIMS dataset shows there are no formal flood defences within the vicinity of the site.	
Emergency planning		
Flood warning	<p>The eastern and southeastern boundaries of the site have been identified to be located within an area of flood alerts for the River Clyst, Culm and their tributaries.</p> <p>The eastern boundary of the site is also located within the River Clyst from Broadclyst to Clyst St Mary flood warning area.</p> <p>Mapping: Brcl_31 - Flood Warnings and Alerts</p>	

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Access and egress	Access and egress is shown to be largely unaffected during all assessed events, with depths of up to 0.34m in a small, localised area on Mosshayne Lane to the west of the site shown in the 1% AEP plus climate change surface water modelling.	
Requirements for drainage control and impact mitigation		
Broad-scale assessment of possible SuDS	<p>Geology and Soils</p> <p>The geology consists of interbedded sandstone and conglomerate, with mudstone, siltstone and sandstone to the northwest of the site. The southwestern portion of the site contains no superficial deposits, whilst the northeast consists of clay, silt and sandstone. The soils are shown to be predominantly freely draining slightly acid sandy soils, with slightly acid loamy and clayey soils with impeded drainage to the north. A small section to the far north adjacent to the River Clyst is shown to be loamy and clayey floodplain soils with naturally high groundwater. This suggests that infiltration is unlikely to be a viable means of surface water disposal in the north but may be viable in the south. However, the infiltration potential of the site should be confirmed through infiltration testing, in line with BRE 365.</p> <p>SuDS</p> <ul style="list-style-type: none"> • The site is also located within a nitrate vulnerable zone. Therefore, early engagement with the LLFA and the EA is recommended to determine requirements for the site to manage the impact to surrounding watercourses. Consideration of water quality is likely to be of high importance and demonstrated through the use of the Simple Index Approach. • The site has not been identified to be located within a historic landfill site or a groundwater Source Protection Zone. • Groundwater levels on site are extremely varied, and infiltration on site may be unlikely in some areas to the north due to impeded drainage from clayey soils, and the steep topography. The infiltration potential of the site should be confirmed through infiltration testing, in line with BRE 365. Offsite discharge may therefore be required to discharge surface water runoff. • Surface water discharge rates should not exceed pre-development discharge rates for the site and should be designed to be as close to greenfield runoff rates as reasonably practical in consultation with the LLFA. It may be possible to reduce site runoff by maximising the permeable surfaces on site using a combination of permeable surfacing and soft landscaping techniques. • Drainage should mimic existing catchments. SuDS measures should follow the discharge hierarchy, and if it is proposed to discharge runoff to a watercourse or sewer system, the condition and capacity of the receiving watercourse or asset should be confirmed through surveys and the discharge rate agreed with the asset owner. • Due to the topography, any surface water not intercepted via infiltration will drain via gravity to the north/eastern boundary towards the River Clyst, or into the small tributary in the southwestern corner. It is therefore recommended that the LLFA and the EA are consulted about 	

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	viable discharge locations for surface water from the site and their attenuation potential.	
Opportunities for wider sustainability benefits and integrated flood risk management	<ul style="list-style-type: none"> • Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could also provide wider sustainability benefits to the site and surrounding area. Proposals to use SuDS techniques should be discussed with relevant stakeholders (LPA, LLFA and EA) at an early stage to understand possible constraints. • The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development. • Opportunities to incorporate source control techniques such as green roofs, permeable surfaces and rainwater harvesting must be considered in the design of the site. • SuDS are to be designed so that they are easy to maintain, and it should be set out who will maintain the system, how the maintenance will be funded and should be supported by an appropriately detailed maintenance and operation manual. • SuDS should be designed with a holistic approach, combining ecology, landscape and drainage requirements specific to the site, incorporating Biodiversity Net Gain requirements. • Opportunities to incorporate filtration techniques such as filter strips, filter drains and bioretention areas must be considered. Consideration should be made to the existing condition of receiving waterbodies and their Water Framework Directive objectives for water quality, along with the location of the site in a Nitrate Vulnerable Zone. The use of multistage SuDS treatment will improve water quality of surface water runoff discharged from the site and reduce the impact on receiving water bodies. • The potential to utilise conveyance features such as swales to intercept and convey surface water runoff should be considered. Conveyance features should be located on common land or public open space to facilitate ease of access. • SuDS should be designed in line with Devon County Councils SuDS Guidance. https://www.devon.gov.uk/floodriskmanagement/document/sustainable-drainage-system-guidance-for-devon-2023/#dcc-documents-cpt-contents 	
NPPF and planning implications		
Exception Test requirements (Local Authority considerations)	<p>The Local Authority will need to confirm that the Sequential Test has been carried out in line with national guidelines. The Sequential Test will need to be passed before the Exception Test is applied.</p> <p>The NPPF classifies the usage as “More Vulnerable”; this type is taken into consideration for the Exception Test.</p> <p>The site is partially located within Flood Zone 2 and 3, and the 0.1% AEP surface water extent, however providing development is proposed outside of the areas at risk (within Flood Zone 1), the Exception Test is not required for this site. Should development be proposed within Flood Zone 2 or 3, the</p>	



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individual plots. It should also be noted that the site should not discharge surface water into combined sewer.

- Infiltration rates are assessed on site as part of a drainage strategy.
- There is early engagement with the LLFA and the EA on the proposed SuDS measures and infiltration rate to discuss requirements on the site meeting relevant conditions due to the sites location within a nitrate vulnerable zone.
- Cumulative Impact Assessment policy documents must be understood, and the cumulative impact of development should be considered.