East Devon District Council Level 2 Strategic Flood Risk Assessment Detailed Site Summary Tables

Site Code	**Otry_10**
Address	Land south of Strawberry Lane, Ottery St Mary
Area	1.4 hectares
Current land use	Greenfield
Proposed land use	Housing and mixed use
Flood Risk Vulnerability	More vulnerable
Sources of flood risk	
Location of site	The site is located to the southwest of Ottery St Mary, south of Strawberry Lane and west of Ottery St Mary Bowling Club. The site is split into two parcels of land to the north and south. The south of the site is adjacent to an unnamed tributary of the River Otter, which joins the River Otter approximately 430m east of the site.
Topography	The Environment Agency's 1m resolution 2022 Composite LiDAR shows that the topography of the site declines from the west (46.5mAOD) to the east (40mAOD), with a continuous gradient. The gradient is approximately 8%, therefore the site is considered to have a gentle slope. There is potential some ground alterations may be required to accommodate any proposed SuDS features and the effect on infiltration and any proposed SuDS features should be investigated by the developer. The catchment therefore drains east into the River Otter.
Existing drainage features	No existing drainage features have been identified on site, with the exception of the watercourse to the east and south of the site.
Fluvial	**Available data and mapping:** Environment Agency Flood Map for Planning for Rivers and Sea. **Data analysis:** Details of the sites location within each Flood Zone are provided within the SFRA Site Screening Appendix. **Flood characteristics:** The site is partially located within Flood Zone 2 and 3 to the south of the site, and Flood Zone 2 to the northeast. The remainder (the majority) of the site is located within Flood Zone 1. • Flood Zone 1 represents areas which have less than 1 in 1000 (0.1%) chance of river flooding in a given year. • Flood Zone 2 represents areas which have less than 1 in 100 (1%) but greater than 1 in 1000 (0.1%) chance of river flooding in a given year. • Flood Zone 3 representing an area greater than 1 in 100 (1%) chance of river flooding in a given year. As there is no detailed modelling available for this site, Flood Zone 3a has been used as a proxy for Flood Zone 3b. Detailed modelling should be undertaken as part of a detailed site-specific Flood Risk Assessment to define the extent of Flood Zone 3b. In the absence of detailed modelling, the Risk of Flooding from Surface Water dataset has been used to assess the depth, hazard and velocity flood risk to

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	the site, as extents are shown to be similar to the Flood Zones. Consideration should still be given to the Flood Zones and detailed modelling may be required within a site specific assessment.
Fluvial plus climate change	In the absence of detailed modelling, the Risk of Flooding from Surface Water dataset with a climate change allowance has been used to assess the depth, hazard and velocity flood risk to the site, as extents are shown to be similar to the Flood Zones. Consideration should still be given to the Flood Zones and detailed modelling may be required within a site specific assessment.
Surface Water	**Available data and mapping:** Environment Agency's Risk of Flooding from Surface Water dataset for the 3.33%, 1% and 0.1% AEP events. Otry_10 - Surface Water 3.33% AEP – Depth Otry_10 - Surface Water 3.33% AEP – Hazard Otry_10 - Surface Water 3.33% AEP - Velocity Otry_10 - Surface Water 1% AEP – Depth Otry_10 - Surface Water 1% AEP – Hazard Otry_10 - Surface Water 1% AEP - Velocity Otry_10 - Surface Water 0.1% AEP – Depth Otry_10 - Surface Water 0.1% AEP – Hazard Otry_10 - Surface Water 0.1% AEP - Velocity **Data analysis:** **3.3% AEP (1 in 30 year) event:** Proportion - <1% Max Depth – 0.17m — Mean Depth – 0.17m Max Velocity – 0.23m/s — Mean Velocity – 0.16m/s Max Hazard – 0.6 — Mean Hazard – 0.59 **1% AEP (1 in 100 year) event:** Proportion - 3% Max Depth – 0.22m — Mean Depth – 0.12m Max Velocity – 1.29m/s — Mean Velocity – 0.5m/s Max Hazard – 0.71 — Mean Hazard – 0.61 **0.1% AEP (1 in 1000 year) event:** Proportion - 9% Max Depth – 0.43m — Mean Depth – 0.2m Max Velocity – 1.94m/s — Mean Velocity – 1.0m/s Max Hazard – 1.55 — Mean Hazard – 0.94 **Flood characteristics:** The site is shown to be at risk of flooding in the 3.3% AEP event with less than 1% at risk to the south of the site, and a mean/max depth of 0.17m. The extent of flooding increases in the southern portion of the site across the 1% and 0.1% AEP events. It is evident that as the flooding extent increases from the 3.3% AEP to the 1% AEP events, the average flood depth decreases. The mean velocity and hazard during the 0.1% AEP events are shown to be 1m/s and 0.94 rated as a 'danger to some'.
Surface water plus climate change	**Available data and mapping:** Environment Agency's Risk of Flooding from Surface Water dataset for the 3.33%, 1% and 0.1% AEP events with 65% Climate Change scenarios. Otry_10 - Surface Water 3.33% AEP plus 65% Climate Change – Depth

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	Otry_10 - Surface Water 3.33% AEP plus 65% Climate Change – Hazard Otry_10 - Surface Water 3.33% AEP plus 65% Climate Change - Velocity Otry_10 - Surface Water 1% AEP plus 65% Climate Change – Depth Otry_10 - Surface Water 1% AEP plus 65% Climate Change – Hazard Otry_10 - Surface Water 1% AEP plus 65% Climate Change - Velocity Otry_10 - Surface Water 0.1% AEP plus 65% Climate Change – Depth Otry_10 - Surface Water 0.1% AEP plus 65% Climate Change – Hazard Otry_10 - Surface Water 0.1% AEP plus 65% Climate Change - Velocity **Management Catchment:** Otry_10 is located within the East Devon Management Catchment. The Environment Agency guidance recommends that the Upper End allowance is considered for both the 3.3% and 1% AEPs for the 2070's epoch, unless the allowance for the 2050's epoch is higher, in which case this should be used. This is appropriate for development with a lifetime beyond 2100. The recommended uplift on peak rainfall intensity for the 3.3% AEP is 40% and for the 1% AEP is 45%. As Risk of Flooding from Surface Water data with a 65% uplift was already available this has been used as the best available data for the 3.3%, 1% and 0.1% AEPs. **Data analysis:** **3.3% AEP (1 in 30 year) plus 65% climate change event:** Proportion - 6% Max Depth – 0.31m Mean Depth – 0.14m Max Velocity – 1.72m/s Mean Velocity – 0.76m/s Max Hazard – 1.3 Mean Hazard – 0.7 **1% AEP (1 in 100 year) plus 65% climate change event:** Proportion - 8% Max Depth – 0.42m Mean Depth – 0.2m Max Velocity – 1.97m/s Mean Velocity – 1.02m/s Max Hazard – 1.55 Mean Hazard – 0.91 **0.1% AEP (1 in 1000 year) plus 65% climate change event:** Proportion - 13% Max Depth – 0.74m Mean Depth – 0.34m Max Velocity – 2.96m/s Mean Velocity – 1.59m/s Max Hazard – 2.52 Mean Hazard – 1.53 **Flood characteristics:** The site is shown to be at risk of flooding in all three scenarios to the south of the site with an average depth in the 0.1% AEP plus 65% climate change of 0.34m. The average velocity is shown to be 1.59m/s, with a maximum of 2.96m/s. The average hazard rating is shown to be a 'danger to most' at 1.53, however this only relates to a small proportion of the site, as only 13% of the site is at risk of flooding in the 0.1% AEP plus climate change event.
Reservoir	The site is not located near to a Wet or Dry day reservoir flooding extent, according to the Environment Agency's reservoir flood mapping.
Groundwater	**Available data and mapping:** The JBA Groundwater Flood Data Map (GW5) is provided as a 5m resolution grid. Otry_10 - Groundwater Emergence **Flood characteristics:** During a 1% AEP groundwater flood event, groundwater levels on site are predominantly either at or very near (within

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	0.025m of) the ground surface, with a small portion to the west of the site between 0.025m and 0.5m below the ground surface. Flow paths would be expected to follow the topography of the site and be similar to surface water flow paths. The risk of groundwater flooding will require further consideration in a site-specific flood risk assessment.
Sewers	No evidence of sewer flooding has been identified at or near the development site.
Flood history	**Available data and mapping:** Environment Agency's Recorded Flood Outlines. **Flood characteristics:** A small portion to the south of the site is shown to be located within the Environment Agency's Recorded Flood Outlines extent. The channel was shown to exceed capacity (due to no raised defences), in October 2008. There are 6 flooding incidents within Devon County Council's dataset recorded within 100m of the site. All events were recorded in October 2008 from surface water flooding.
Policy zones	
Critical drainage areas	The site has not been identified to be located within a critical drainage area. Mapping: Otry_10 - Critical Drainage Area
Coastal change management areas	The site has not been identified to be located within a coastal change management area.
Flood risk management infrastructure	
Existing defences	The Environment Agency's AIMS dataset shows there are no formal flood defences within the vicinity of the site.
Emergency planning	
Flood warning	A small area to the south and northeast of the site has been identified to be located within an area of flood alerts for the Rivers Otter and Sid, and the Exmouth area. This small area of the site is also within the River Otter (Lower) from Salston to Budleigh Salterton flood warning area. Mapping: Otry_10 - Flood Warnings and Alerts
Access and egress	Access and egress is shown to be largely unaffected during all assessed events, with depths of up to 0.07m along Strawberry Lane shown in the 1% AEP plus climate change surface water modelling. Access and egress from the southern parcel of land is available with depths predominantly less than 0.3m along the lane to the east of the site off Strawberry Lane, however this should be assessed within a site-specific FRA.

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Requirements for drainage control and impact mitigation	
Broad-scale assessment of possible SuDS	**Geology and Soils** The geology consists of interbedded sandstone and conglomerate, with clay, silt and sandstone superficial deposits covering the majority of the site. The soils are shown to be freely draining slightly acid loamy soils. This suggests that infiltration is likely to be a viable means of surface water disposal. **SuDS** • The site is located within a Nitrate Vulnerable Zone. Therefore, early engagement with the LLFA and the EA is recommended to determine requirements for the site to manage the impact to surrounding watercourses. Consideration of water quality is likely to be of high importance and demonstrated through the use of the Simple Index Approach. • The site has not been identified to be located within a groundwater source protection zone or historic landfill site. • Groundwater levels on site suggest that levels are close to the ground surface, which may limit infiltration. The infiltration potential of the site should be confirmed through infiltration testing, in line with BRE 365. Offsite discharge may therefore be required to discharge surface water runoff. • Surface water discharge rates should not exceed pre-development discharge rates for the site and should be designed to be as close to greenfield runoff rates as reasonably practical in consultation with the LLFA. It may be possible to reduce site runoff by maximising the permeable surfaces on site using a combination of permeable surfacing and soft landscaping techniques. • SuDS measures should follow the discharge hierarchy, and if it is proposed to discharge runoff to a watercourse or sewer system, the condition and capacity of the receiving watercourse or asset should be confirmed through surveys and the discharge rate agreed with the asset owner. • Due to the steep topography, any surface water not intercepted via infiltration will drain via gravity to the east of the site and likely drain into the ordinary watercourse to the south of the site. It is therefore recommended that the LLFA and the EA are consulted about viable discharge locations for surface water from the site and their attenuation potential.
Opportunities for wider sustainability benefits and integrated flood risk management	• Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could also provide wider sustainability benefits to the site and surrounding area. Proposals to use SuDS techniques should be discussed with relevant stakeholders (LPA, LLFA and EA) at an early stage to understand possible constraints. • The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development, and the critical drainage area requirements. • Opportunities to incorporate source control techniques such as green roofs, permeable surfaces and rainwater harvesting must be considered in the design of the site.

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	• SuDS are to be designed so that they are easy to maintain, and it should be set out who will maintain the system, how the maintenance will be funded and should be supported by an appropriately detailed maintenance and operation manual. • SuDS should be designed with a holistic approach, combining ecology, landscape and drainage requirements specific to the site, and incorporating Biodiversity Net Gain requirements. • Opportunities to incorporate filtration techniques such as filter strips, filter drains and bioretention areas must be considered. Consideration should be made to the existing condition of receiving waterbodies and their Water Framework Directive objectives for water quality, along with the location of the site in a Nitrate Vulnerable Zone. The use of multistage SuDS treatment will improve water quality of surface water runoff discharged from the site and reduce the impact on receiving water bodies. • The potential to utilise conveyance features such as swales to intercept and convey surface water runoff should be considered. Conveyance features should be located on common land or public open space to facilitate ease of access. • SuDS should be designed in line with Devon County Councils SuDS Guidance. https://www.devon.gov.uk/floodriskmanagement/document/sustainable-drainage-system-guidance-for-devon-2023/#dcc-documents-cpt-contents
NPPF and planning implications	
Exception Test requirements **(Local Authority considerations)**	The Local Authority will need to confirm that the Sequential Test has been carried out in line with national guidelines. The Sequential Test will need to be passed before the Exception Test is applied. The NPPF classifies the usage as "More Vulnerable"; this type is taken into consideration for the Exception Test. The site is partially located within Flood Zone 2 and 3, and the 0.1% AEP surface water extent. During a 1% AEP groundwater flood event, groundwater levels on site are predominantly either at or very near (within 0.025m of) the ground surface. Flow paths would be expected to follow the topography of the site and be expected to be similar to surface water flow paths. Providing development is proposed to the north/west of the site (outside of the areas at risk of fluvial or surface water flooding), the Exception Test is not required for this site. Should development be proposed within Flood Zone 2 or 3, the Exception Test will be required and detailed flood modelling should be undertaken during a site-specific FRA.
Requirements and guidance for site-specific Flood Risk Assessment **(Developer considerations)**	**Flood Risk Assessment:** The Level 1 SFRA has more guidance on the requirements for site specific Flood Risk Assessments and relevant policies and information applicable to development within East Devon District Council. • Consultation with the East Devon District Council, and where relevant South West Water, Devon County Council, and the Environment Agency should be undertaken at an early stage.

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	• Developers should consult with South West Water to ensure that the development aims to help achieve the targets of the Drainage and Wastewater Management Plan. • Development plans should use the Level 1 SFRA for East Devon District Council, as well as the Local Flood Risk Management Strategies to identify cumulative flood risk issues. It should also promote an integrated approach to water management. • The site is located within a medium risk Cumulative Impact Assessment (CIA) catchment and therefore specific CIA policy documents are applicable to this site. **Guidance for site design and making development safe:** • The developer will need to show, through an FRA, that future users of the development will not be placed in danger from flood hazards throughout its lifetime. It is for the applicant to show that the development meets the objectives of the NPPF's policy on flood risk. For example, how the operation of any mitigation measures can be safeguarded and maintained effectively through the lifetime of the development. (Para 048 Flood Risk and Coastal Change PPG). • The risk from surface water flow routes should be quantified as part of a site-specific FRA, including a drainage strategy, so runoff magnitudes from the development are not increased by development across any ephemeral surface water flow routes. A drainage strategy should help inform site layout and design to ensure runoff rates do not exceed greenfield rates. • Arrangements for safe access and egress are likely to be possible, however these will need to be considered further within a site-specific FRA for the surface water events with an appropriate allowance for climate change, using the depth, velocity, and hazard outputs.

Key messages

The site is generally identified to be at low risk, with a small portion of land to the south that is unlikely to be developable. Development is likely to progress if:

- A site-specific FRA is undertaken to assess the risk of fluvial, surface water and groundwater flooding in relation to the proposed development, and the access and egress arrangements.
- Development is placed outside of the areas at risk from surface water and fluvial flooding, within Flood Zone 1. Should development be proposed within areas at risk detailed flood modelling should be undertaken within a site-specific FRA. Any development within Flood Zone 3 should be allocated as an undeveloped open space corridor, and not as gardens, car parking or other features associated with individual plots.
- Infiltration rates are assessed on site as part of a drainage strategy.
- There is early engagement with the LLFA and the EA on the proposed SuDS measures and infiltration rate to discuss requirements on the site meeting relevant conditions due to the sites location within a Nitrate Vulnerable Zone.
- Cumulative Impact Assessment policy documents must be understood, and the cumulative impact of development should be considered.